

Welcome to the University of Vermont Bioretention Laboratory

Amanda Cording, PhD



PhD Dissertation: Evaluating Stormwater Pollutant Removal Mechanisms by Bioretention in the Context of Climate Change

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Committee Member: Dr. Arne Bomblies

















Low Impact Design & Development

LID is an approach to development that aims to mimic pre-development hydrology and uses ecological engineering to remove pollutants in stormwater, for re-use and/or replenishment of groundwater supplies.

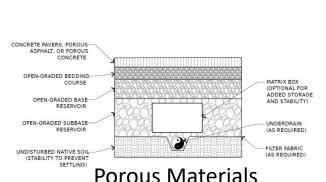
LID uses Green Stormwater Infrastructure (GSI) as a tool.

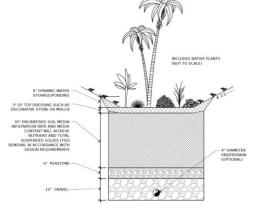


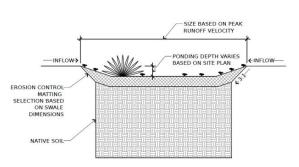












Bioretention "Green Streets"

Vegetated Swales

Many National Champions of Low Impact Development













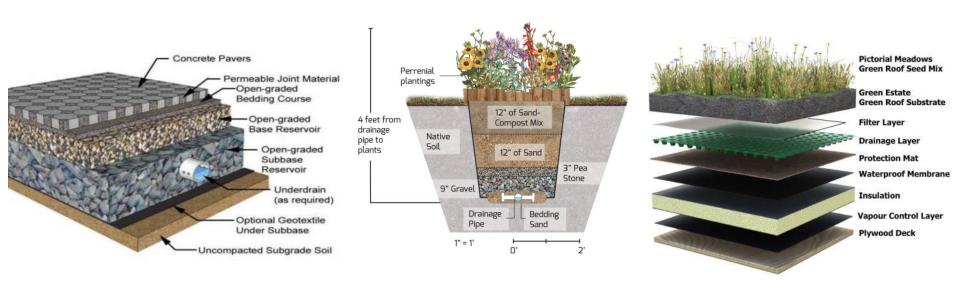




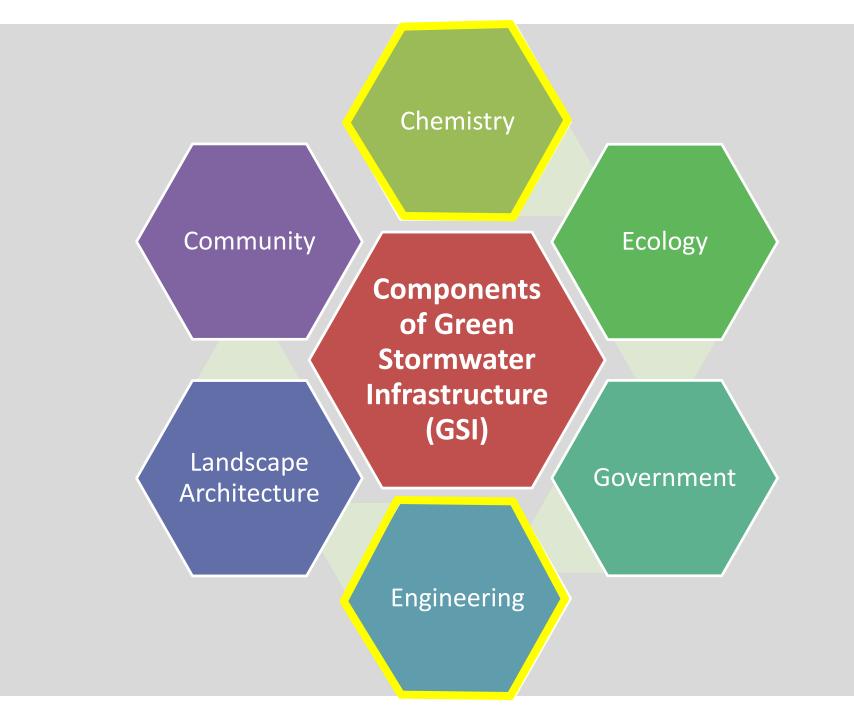




Green Stormwater Infrastructure (GSI)







EPA National Green Infrastructure Strategic Agenda 2013





Green Infrastructure Strategic Agenda 2013

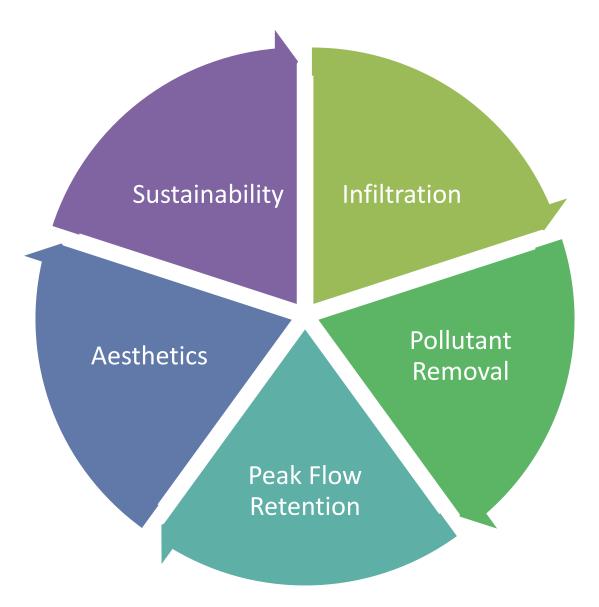
IIS Environmental Protection Agency

Photos courtesy of Abby Hall, EPA

National Objectives:

- 1. Increase federal coordination
- 2. Expand Clean Water Act regulatory support
- 3. Strengthen research and information exchange
- 4. Distribute funding and financing
- 5. Build local capacity

Goals of Bioretention



What is Bioretention?

Reference	Definition
Davis, A. P., Shokouhian, M., Sharma, H., & Minami, C. (2001). Laboratory Study of Biological Retention of Urban Stormwater. Water Environment Research, 73(1), 5–14.	 Layers of soil, mulch, and a variety of plant species. Soil: high sand content to provide rapid infiltration but with low levels of silt and clay Covered with thin layer of wood mulch to prevent erosion and protect the soil layer from drying.
Vermont Agency of Natural Resources. (2002). The Vermont Stormwater Management Manual Volume I - Stormwater Treatment Standards (Vol. I).	 Shallow depression that treats stormwater as it flows through a soil matrix, and is returned to the storm drain system
Collins, K. et al., (2010). Opportunities and challenges for managing nitrogen in urban stormwater: A review and synthesis. <i>Ecological Engineering</i> , 36(11), 1507–1519.	Shallow, vegetated depressions, back- filled with soil filter media that is designed to accept and infiltrate stormwater.

What is Bioretention?

Reference	Definition
Claytor, R. A., & Schueler, T. R. (1996). <i>Design of Stormwater Filtering Systems</i> (pp. 1–220).	The term stormwater filter refers to a diverse spectrum of stormwater treatment methods utilizing various media, such as sand, peat, grass, soil or compost to filter out pollutants entrained in urban stormwater.
Department of Environmental Quality, Michigan. (2008). Low Impact Development Manual for Michigan: A Design Guide for Implementers and Reviewers.	Bioretention soils should be amended with a composted organic material. A recommended range of a soil mixture is 20-40 percent organic material (compost), 30-50 percent sand, and 20-30 percent topsoil, although any soil with sufficient drainage may be used for bioretention.
Washington State University Pierce County Extension. (2012). Low Impact Development Technical Guidance Manual for Puget Sound.	The bioretention soil media (BSM) placed in the cell or swale is typically composed of a highly permeable sandy mineral aggregate mixed with compost .

What is Bioretention?

Definition: bioretention systems are ecological engineered to reduce peak flow rates and volumes while also removing stormwater pollutants through physical, biological, and chemical mechanisms.

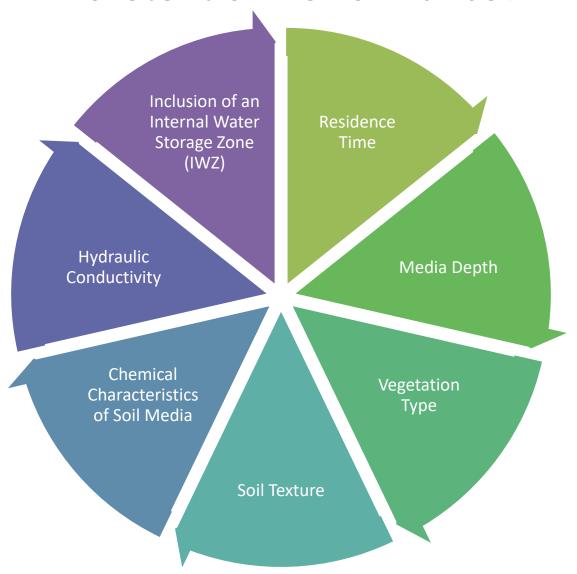






Davis 2008; Dietz and Clausen 2006; Zinger et al. 2013; Collins et al. 2010. Image Credits: Amanda Cording (left, middle) and EcoSolutions (right).

What Design Factors Influence Bioretention Performance?

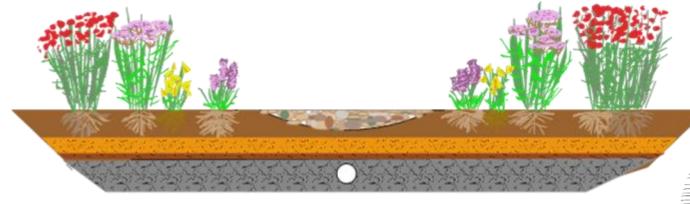


Bioretention: Nutrient Removal

Nutrient removal is extremely variable

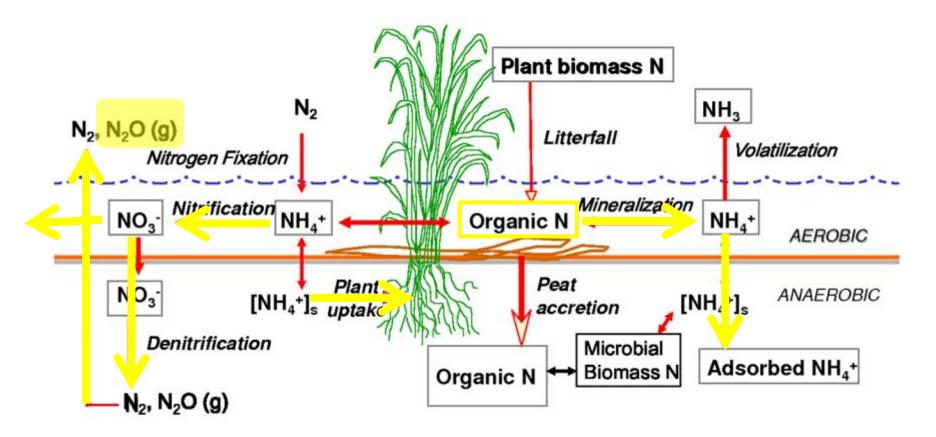
- Labile N (-630% to 98% removal)
- NO_3^- Effluent [] = 10 µg L⁻¹ to 2,100 µg L⁻¹
- Labile P (-78% to 98% removal)
- SRP Effluent [] = $< 10 \mu g L^{-1}$ to 2,200 $\mu g L^{-1}$

*Lake Champlain P Targets: 15 – 40 μg L⁻¹



Davis et al. (2007); Bratieres et al. (2008); Debusk et al. (2011); Dietz and Claussen (2006); Hunt et al. (2006); O'Neill and Davis (2011); Image Credit: Amanda Cording

Nitrogen Removal Mechanisms



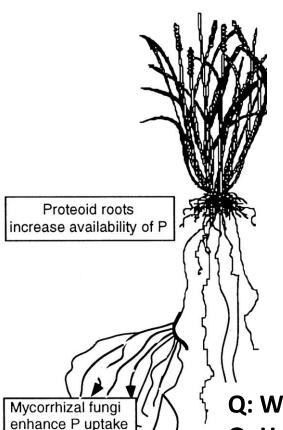
Q: Which mechanisms are dominant in bioretention?

Q: How can we maximize removal through design?

Q: Do the conditions encourage N₂O release or uptake?



Phosphorus Removal Mechanisms



- 1. Physical Filtration: Non-labile P
- 2. Sorption of SRP: Fe, Ca, and Al in Soil

$$2 = \text{FeOH}^{-0.5} + \text{PO}_4^{3-} + 2\text{H}^+ = (=\text{FeO})_2 \text{PO}_2^{2-} + 2\text{H}_2\text{O}$$

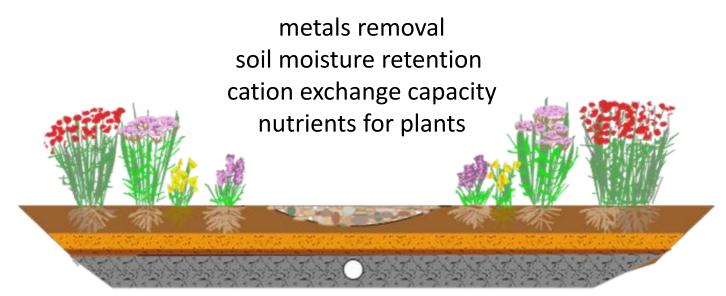
3. Plant Uptake: SRP

Q: Which mechanisms are dominant in bioretention?

Q: How can we maximize removal mechanisms through design?

Inconsistent P Removal in Bioretention

- Some of the variability is thought to be attributed to the soil media selected
- Sand based bioretention soil designs are common
- Organic amendments (compost, mulch) are widely recommended to provide:



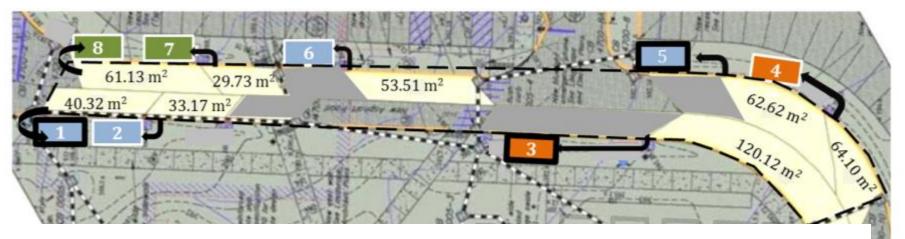
Bratieres et al. 2008; DeBusk and Wynn 2011; Michigan Department of Environmental Quality 2008; Thompson et al. 2008; Vermont Agency of Natural Resources 2002; Washington State University Pierce County Extension 2012.

Research Site: University of Vermont Outdoor Bioretention Laboratory



- Constructed in November of 2012
- Total area: approx.
 5,000 ft² or 0.1 acres
- Eight small paved road sub-watersheds
- Bioretention Surface Areas: 29.73 m² to 120.12 m²

Research Site







Bioretention Layout View

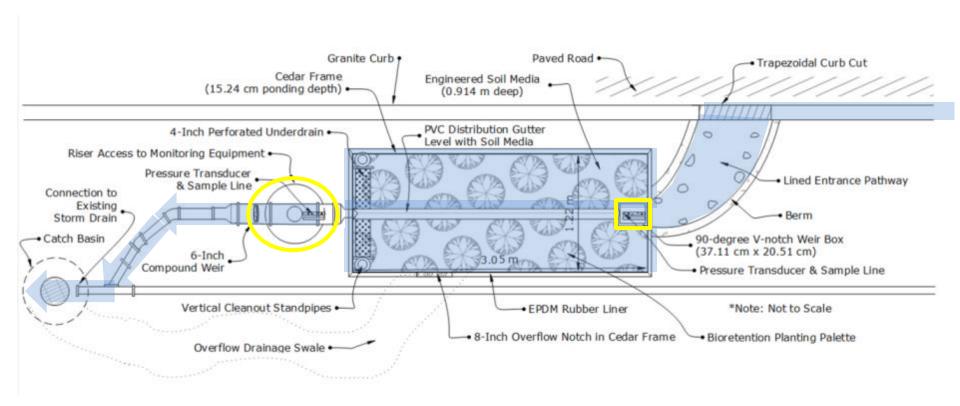


Image Reference: Cording, A., Hurley, S., Whitney, D. (**Submitted**) Monitoring methods and designs for evaluating bioretention performance. Journal of Environmental Engineering.

Monitoring Objectives:

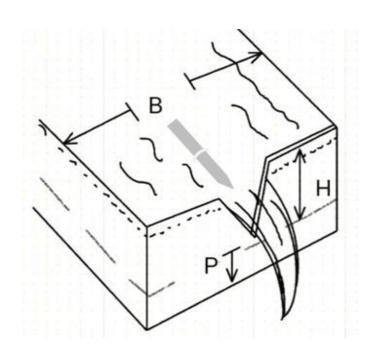
Characterize stormwater mass loads from small paved road watersheds throughout the inflow and outflow hydrograph



How do you measure the runoff from the road surface?



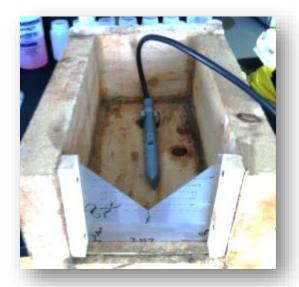
Weir thickness = 1.59 mm stainless steel
Teledyne™ ISCO Model 720 Pressure Transducer



Maximum Capacity = 10.05 L

ASTM –D5242; U.S. Bureau of Reclamation (2001)

Monitoring Bioretention Systems



Inflow 90° Weir Box



Outflow Thel-Mar™ Weir

 $Q = CH^n$

Where:

Q = flow rate over the weir (cfs, L s⁻¹)

C= coefficient of discharge, or weir coefficient

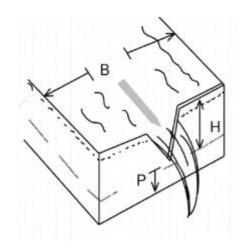
H= height of water behind the weir (pressure transducer)

n = an empirical exponent (dimensionless)

Inflow Monitoring: Weir Rating Curve

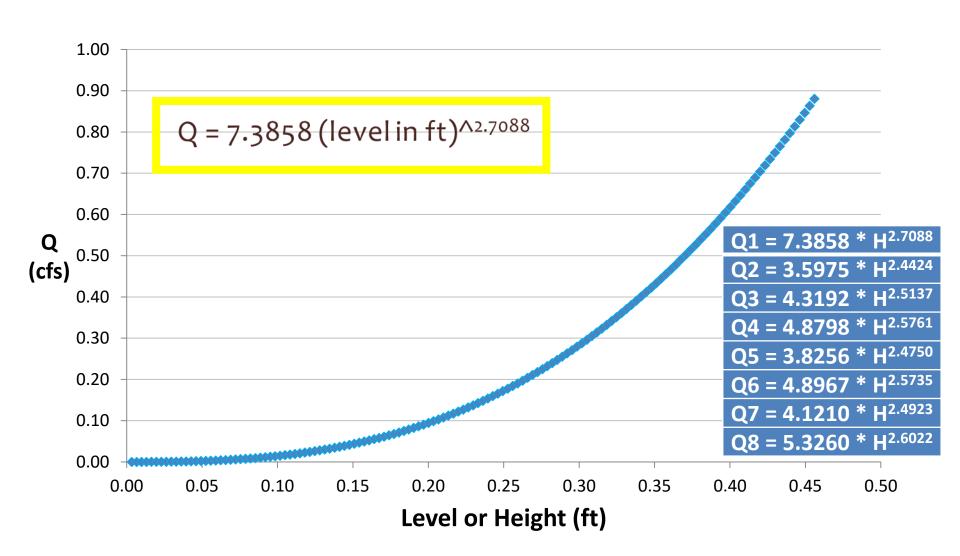
$$Q=CH^n \rightarrow log Q = log C + n log H \rightarrow log Q = n log H + log C$$



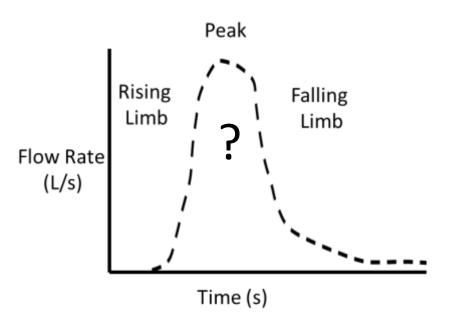


- 1. $logQ = nlogH + logC \rightarrow y = mx + b$, to get the values of C and n
- 2. Plot Q (Y-axis) and H (x-axis) on a log-log plot
- 3. The equation of the line contains weir coefficient and exponent

Developing a Weir Rating Curve



How to Capture the Inflow Hydrograph?



Time-Based Sampling:

- √ Homogeneous paved surface
- ✓ Small watersheds

- 1. Time of concentration (T_c) -> intensity duration frequency (IDF) curve
- 2. Rainfall intensity -> peak discharge with the rational method
- 3. Select the rainfall depth you want to sample (0.9 inches)

Capturing the Inflow Hydrograph: Estimating Time of Concentration

$$Tc = \frac{G (1.1 - C)L^{0.5}}{(100 S)^{1/3}}$$

Where,

T_c is the time of concentration (min)

G is equal to 1.8 (FAA method, constant)

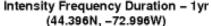
C is the runoff coefficient using the rational method (dimensionless)

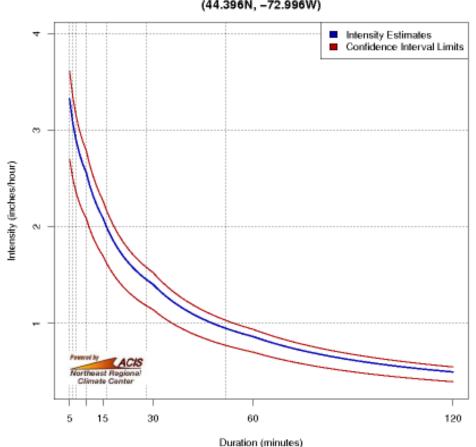
L is the longest distance from the fixed location within the watershed (ft)

S is the slope of the watershed (ft ft⁻¹or m m⁻¹)

 $T_c = 4.73$ minutes to 8.27 minutes

Capturing the Inflow Hydrograph: Estimating Rainfall Intensity with the Intensity Duration Frequency Curve





Rainfall intensity: 3.32 in hr^{-1} (2.34 x 10^{-5} m s^{-1}) to 2.57 in hr^{-1} (1.81 x 10^{-5} m s^{-1})

Northeast Regional Climate Center Precipitation Data

Capturing the Inflow Hydrograph: Estimating Peak Flow Rate using the Rational Method

$$Q = C_f * C_i * A$$

Where,

Q is the peak discharge, or flow rate (ft³ s⁻¹, m³ s⁻¹, L s¹)

C_f is the runoff coefficient (dimensionless)

C_i is the rainfall intensity (ft s⁻¹ or m s⁻¹)

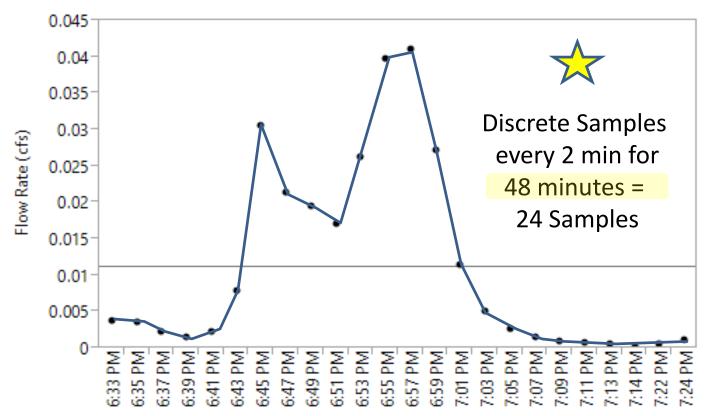
A is the drainage area (ft² or m²)

$$Q_{peak} = 0.02 \text{ to } 0.07 \text{ ft}^3 \text{ s}^{-1}$$

Sampling the Inflow Hydrograph

 $Time = \frac{watershed\ area\ x\ rainfall\ depth}{peak\ flow\ rate}$

Time to Monitor Inflow Hydrograph (0.9 inch) = 34 to 48 minutes



What infrastructure do you need to measure the outflow from bioretention?

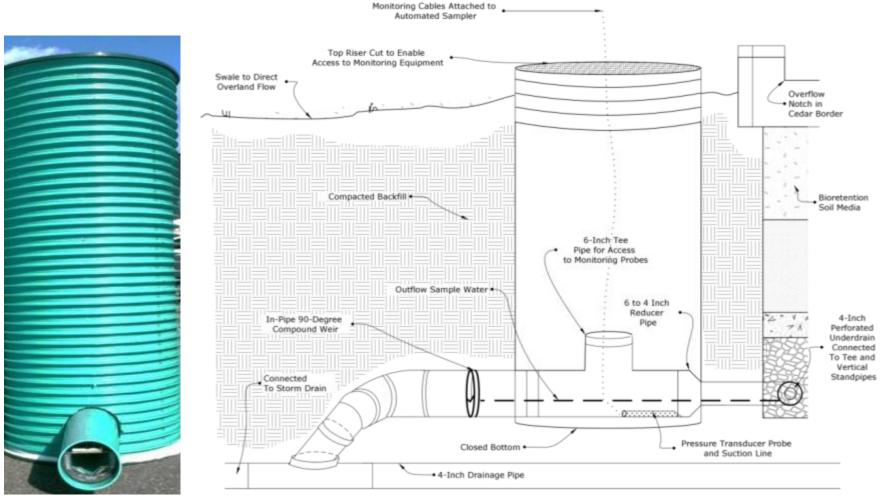
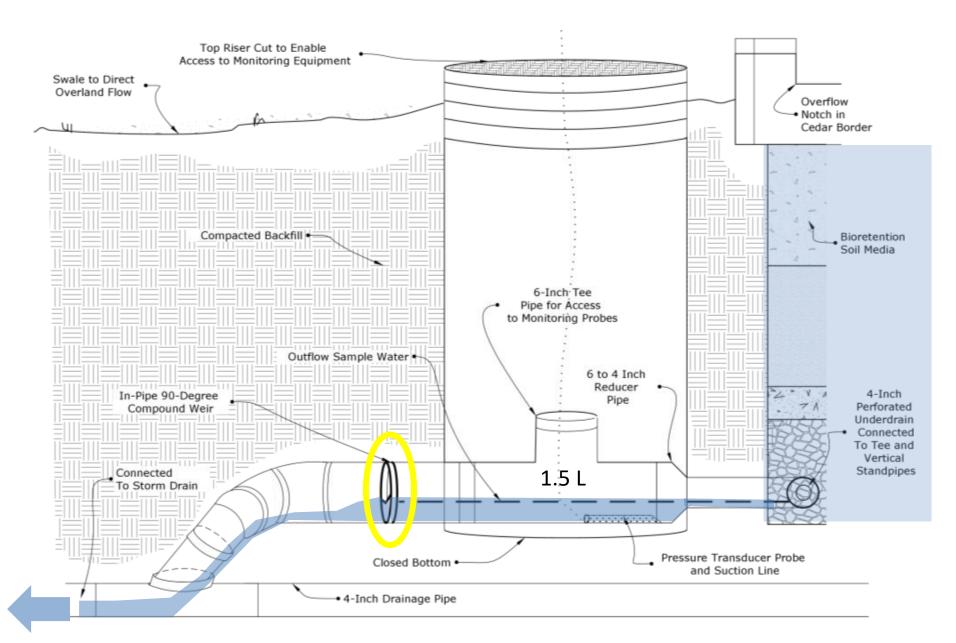


Image Reference: Cording, A., Hurley, S., Whitney, D. (**Submitted**) Monitoring methods and designs for evaluating bioretention performance. Journal of Environmental Engineering.

How to Capture the Outflow Hydrograph?



Capturing the Outflow Hydrograph: Estimating Hydraulic Conductivity

$$K_z = \frac{D}{\sum_{i=1}^n \frac{d_i}{k_i}}$$

Where,

K₂ is the vertical hydraulic conductivity for the layered system (m s⁻¹)

D is the total cumulative depth of the layers (m)

d_i is the depth of a given layer (m)

k_i is the hydraulic conductivity of a given layer (m s⁻¹)

$$K_{x} = \sum_{i=1}^{n} \frac{K_{i} d_{i}}{d}$$

Where,

 K_x is the horizontal hydraulic conductivity (m s⁻¹)

d_i is the depth of a given layer (m)

K_i is the hydraulic conductivity of a given layer (m s⁻¹)

d is the horizontal distance of the given layer (m)

Conventional Bioretention Design

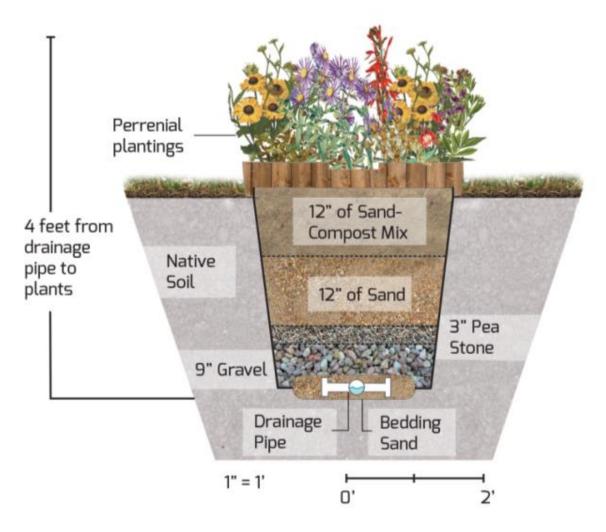


Image Reference: Cording, A., Hurley, S., Adair, E., Ross, D. (2017). Evaluating critical bioretention designs features in the context of climate change. Manuscript in Preparation

Capturing the Outflow Hydrograph: Estimating Hydraulic Conductivity

Bioretention Media	Depth (m)	Hydraulic Conductivity (m s ⁻¹)	d _i /k _i	
Sand/Compost Mixture	0.3048	1.50E-04	2.03E+03	
Medium Sand	0.3048	6.90E-04	4.42E+02	
Pea Gravel	0.0762	6.40E-03	1.19E+01	
Gravel	0.2286	9.14E-03	2.50E+01	
Total $d_i/k_i = 2.51E+03$				
Total Depth = 0.9144 m				
		k	$\zeta_z \text{ (m s}^{-1}\text{)} = 3.64\text{E}-04$	

 $K_7 = 131.04 \text{ cm hr}^{-1} \text{ or } 51.59 \text{ in hr}^{-1}$

Media Infiltration Rates

Reference	Infiltration Rate	
This study	Modelled Rate at Installation: 131 cm hr ⁻¹	
Arias et al (2001)	Actual Rate: 463 cm hr ⁻¹	
Brix et al. (2001)	Actual Rate: 92 cm hr ⁻¹	
Chen et al (2013)	Actual Rate: 1.3 cm hr ⁻¹	
Davis et al. (2009)	Recommends > 2.5 cm hr ⁻¹	
Debusk et al. (2011)	Actual Rate: 11.8 cm hr ⁻¹	
Dietz and Clausen (2005)	Design Rate: 10 – 13 cm hr ⁻¹ . Actual Rate: 3.5 cm hr ⁻¹	
Hatt et al. (2008)	Actual Rate: 26.028 cm hr ⁻¹ to 232.92 cm hr ⁻¹ in	
	different treatments	
Hunt et al. (2006)	Actual Rate: 7.62 cm hr ⁻¹ to 38.1 cm hr ⁻¹	
Li and Davis (2008)	Actual Rate: Reduction from 43 – 164 cm hr ⁻¹ to 3-11	
	cm hr ⁻¹	
Lucas and Greenway (2011)	Vegetated: 27.7 cm hr ⁻¹ to 59.6 cm hr ⁻¹	
Thompson et al. (2008)	Actual Rate: 150 to 178 cm hr ⁻¹ (sand/compost mix)	
Washington State University		
Pierce County Extension	Recommends > 2.54 cm hr ⁻¹	
(2012)		

Capturing the Outflow Hydrograph: Estimating Hydraulic Conductivity

$$K_z = \frac{D}{\sum_{i=1}^n \frac{d_i}{k_i}}$$

Where,

K₂ is the vertical hydraulic conductivity for the layered system (m s⁻¹)

D is the total cumulative depth of the layers (m)

d_i is the depth of a given layer (m)

k_i is the hydraulic conductivity of a given layer (m s⁻¹)

$$K_{x} = \sum_{i=1}^{n} \frac{K_{i} d_{i}}{d}$$

Where,

K_x is the horizontal hydraulic conductivity (m s⁻¹)

d_i is the depth of a given layer (m)

K_i is the hydraulic conductivity of a given layer (m s⁻¹)

d is the horizontal distance of the given layer (m)

Estimating Hydraulic Conductivity

$$T = \frac{A_w D}{K_z A_{BR(z)}} + \frac{A_w D}{K_x A_{BR(\mathbf{x})}}$$

Where,

T is the time for the outflow peak to reach monitoring equipment (s)

A_w is the watershed area (m²)

D is the selected rainfall depth (m)

K_z is the cumulative vertical hydraulic conductivity (m s⁻¹)

 K_x is the horizontal hydraulic conductivity (m s⁻¹)

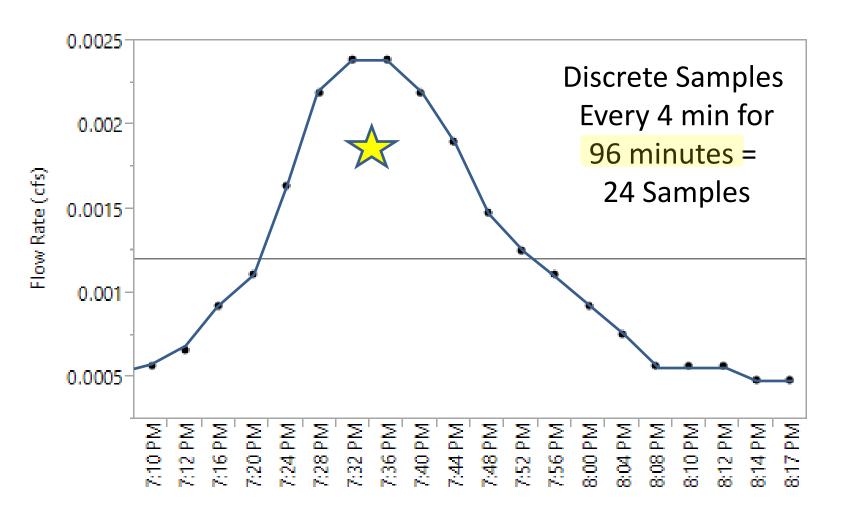
A_{BR} (z) is the vertical cross-sectional area along the Y-plane (m²)

 A_{BR} (x) is the vertical cross-sectional area of the layer directly above the flow impeding layer along the X-plane (m²)

Time (0.9 inch storm) = 50 min + 40 min (inflow runoff travel time) = 90 min

Sampling the Outflow Hydrograph

Time Needed to Monitor Outflow Hydrograph = 90 minutes



Installing Outflow Monitoring Equipment

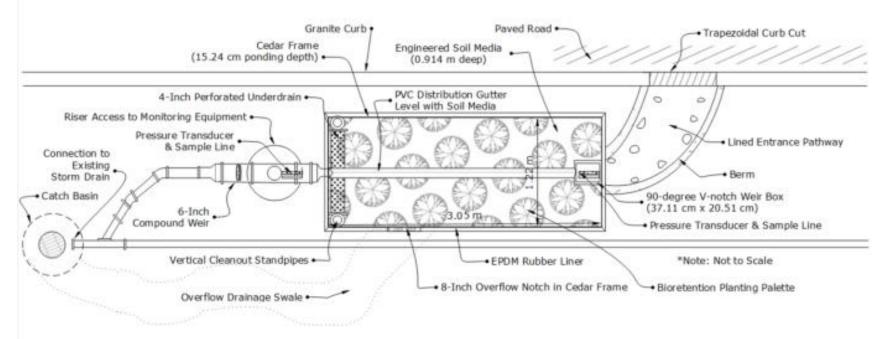




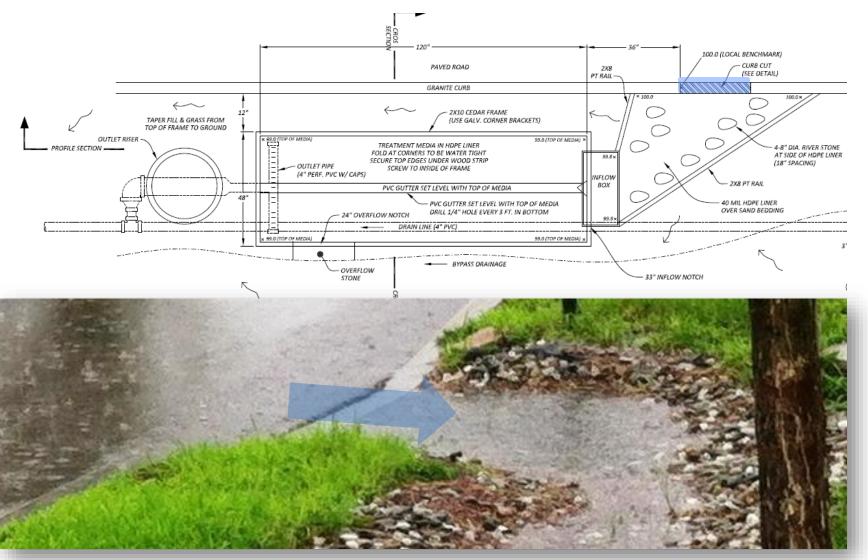
Conclusions: Monitoring Methods and Designs for Evaluating Bioretention Performance

The inflow and outflow monitoring infrastructure/sampling method allowed for:

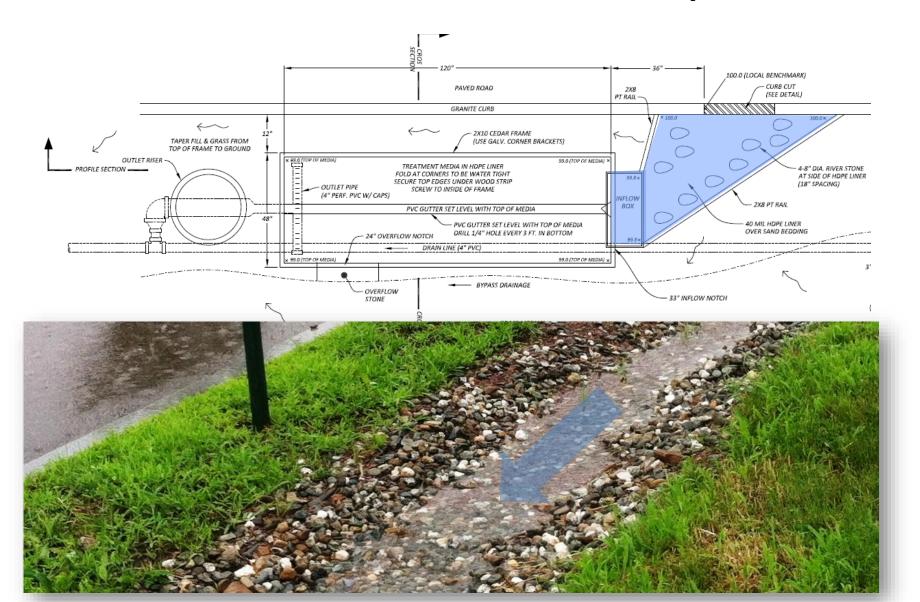
- 1. multiple samples throughout the hydrograph
- 2. conversion of concentration to mass for any sample
- 3. the comparison of pollutant mass from inflow to outflow



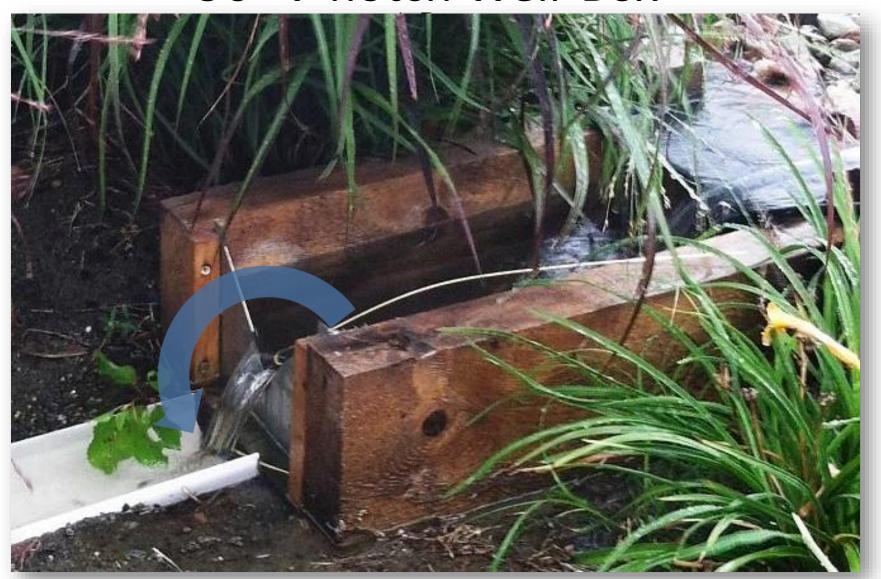
Plan View: Water into Curb Cut



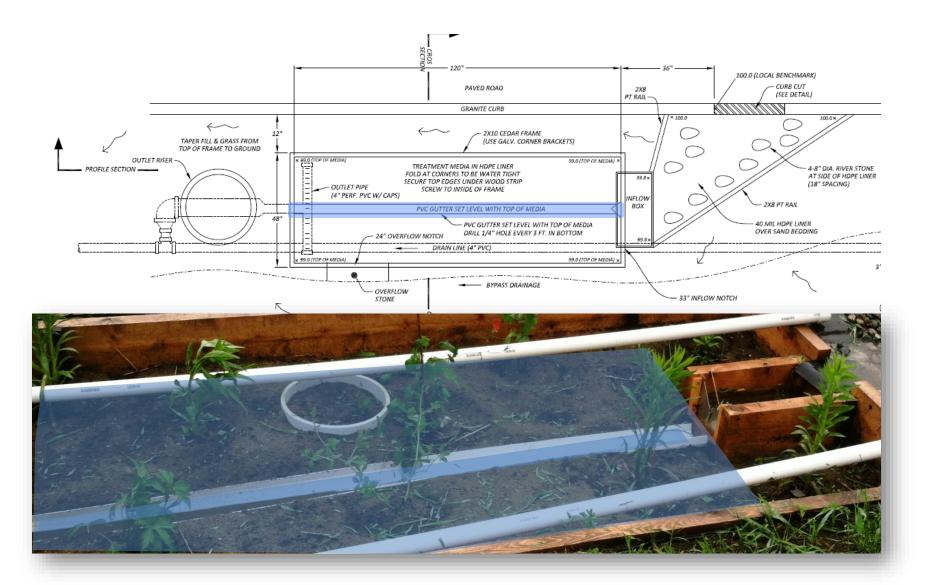
Plan View: Filter Strip



Inflow Monitoring Using 90° V-notch Weir Box

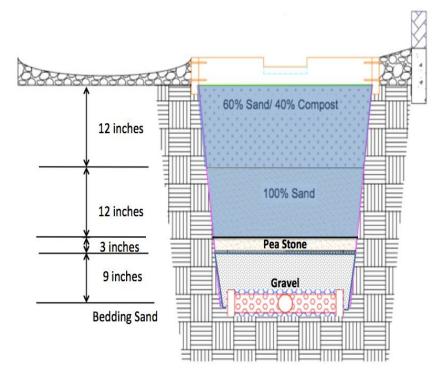


Plan View: Distribution Channel





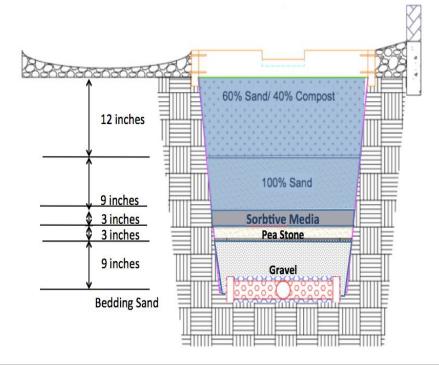
Soil Profile



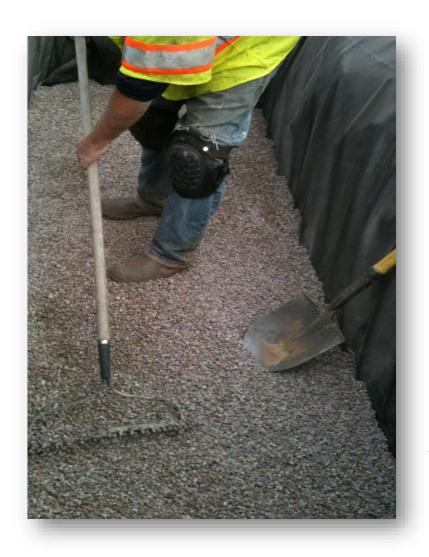
12" 60:40 sand/compost layer



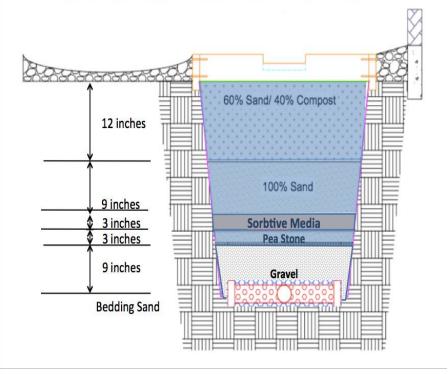
Soil Profile: SorbtiveMedia



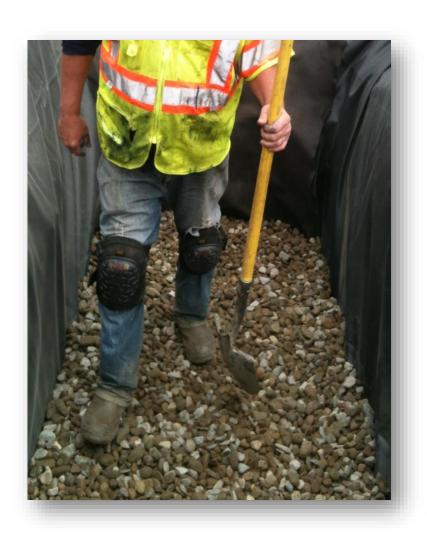
3" Imbrium Sorbtive Media™



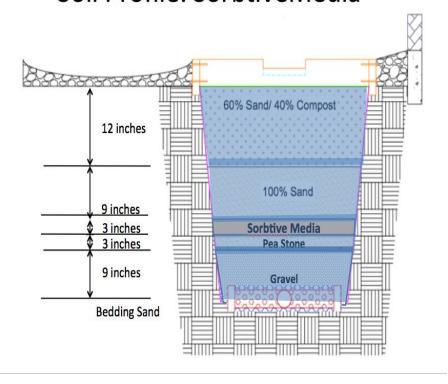
Soil Profile: SorbtiveMedia



3" pea stone layer



Soil Profile: SorbtiveMedia

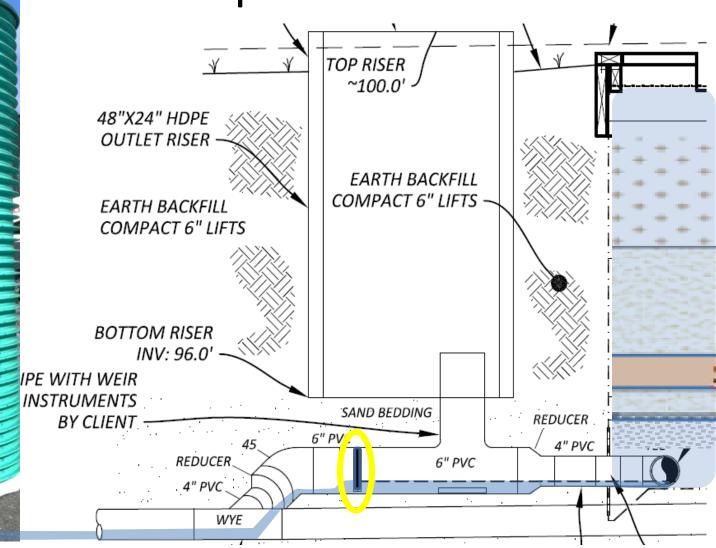


9" gravel layer





Outflow Monitoring Using 6" In-Pipe Thel-Mar ™ Weir



Construction Complete: November 2012



Big thanks to Dave Whitney, EcoSolutions, Andres Torizzo, Watershed Consulting, Imbrium Staff, Arcana, Gardner's Supply, and Tri-Angle Metal Supply

Research Questions

- 1. How do you assess bioretention performance with monitoring?
 - What infrastructure do you need?
 - What sampling regime will capture the hydrograph?
- 2. What pollutant loads are coming off a medium traffic paved road?
 - Do nutrients and sediment mass exhibit a first flush effect?
 - Can we predict the total mass load from a precipitation event?
- 3. How do soil and vegetation influence bioretention performance?
 - Will increased precipitation due to climate change decrease bioretention performance?
 - Are bioretention cells a source or a sink for soil gas emissions?

Testing Bioretention Performance Under Different Conditions

1. Soil Media: Conventional vs. Sorbtive Media™





2. Precipitation: Ambient vs. Increased due to Climate Change (20% increase in CM, 60% increase in SM)





3. Vegetation: Plant Palette 1 vs. Plant Palette 2









Bioretention: Source or Sink for N₂O and CH₄?



Nitrous Oxide (N₂O) Emissions

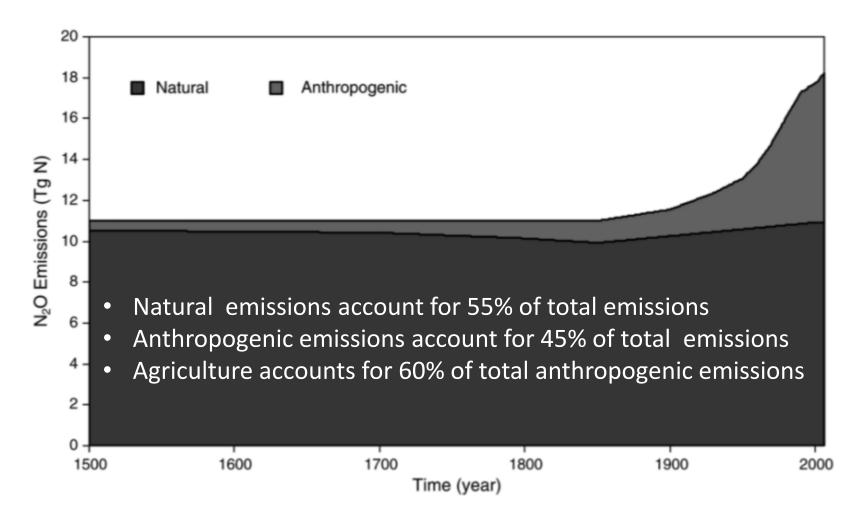
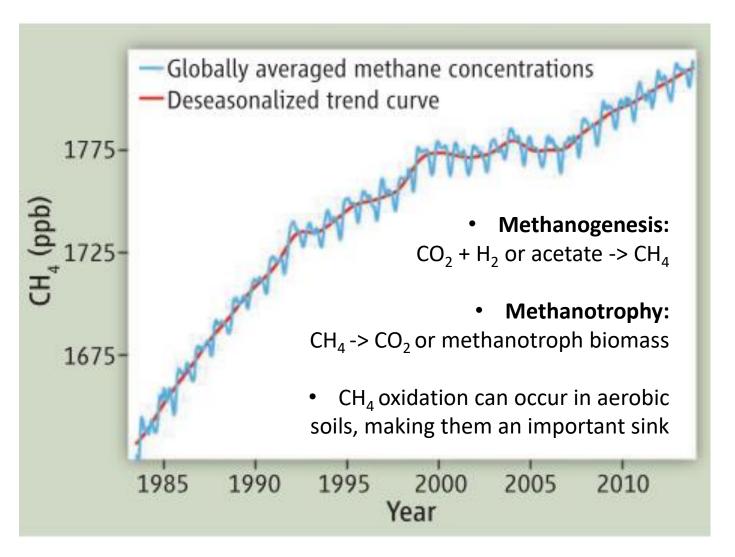


Image Source: Syakila, A., & Kroeze, C. (2011). Matson, P. A., & Harris, R. C. (1995); Firestone and Davidson (1989); Bond-Lamperty and Thomson (2010)

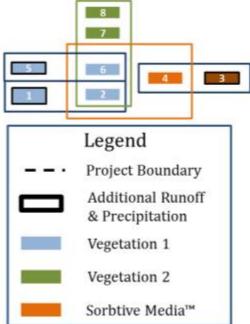
Methane (CH₄) Emissions



Nisbet, E. G., Dlugokencky, E. J., & Bousquet, P. (2014). Atmospheric Science.

Experimental Design and Layout





Bioretention Cells	Paired Treatment	Abbreviation
1 and 2	Conventional Media + 20% vs. Conventional Media	CM20 vs. CM
(2 & 6) and (7 & 8)	Vegetation 1 vs. Vegetation 2	V1 vs. V2
(2 & 6) and 4	Conventional Media vs. Sorbtive Media ™	CM vs. SM
3 and 4	Sorbtive Media™ + Climate 60% Vs. Sorbtive Media™	SM60 vs. SM

Methods: Measuring Stormwater Quality

Equipment	Parameter	Sampling and Analysis Methods
 6700 Series Automatic Samplers (Teledyne™) Model 720 Differential Pressure Transducer 	 TP NLP SRP TN TKN NO₃⁻ TSS Flow Rate 	 Time Based Discrete Samples Based on the Hydrograph Inflow = Every 2 min for 48 min (950 mL) Outflow = Every 4 min for 96 min (500 mL) Inflow to Outflow, 20-L increments (n = 6) Outflow to Outflow, 20-L increments (n = 6) Partial Event Mean Concentration (PEMC)









Methods: Measuring Bioretention Soil Media Characteristics

Equipment	Parameter	Sampling Method
 Soil auger Soil core cylinder Trowel Decagon soil probes 	 NH₄⁺ (n = 13) and NO₃⁻ (n = 13) SRP (n = 7) Bulk Density (n = 11) Ca, K, Mg, Na, S, Mn, Al, Fe, Zn, Cu (n = 7) Cation exchange capacity (CEC) Organic matter content (n = 7) Volumetric water content Electrical conductivity Soil temperature 	 2 M KCl extraction Modified Morgan Change in mass /volume Inductively coupled plasma spectroscopy Ammonium acetate Loss on ignition (375°C) Every five minutes 3 composited sub-samples
		per cell









Methods: Measuring Soil Gas Emissions

Equipment	Parameter	Sampling Method
 Permanent anchors Closed chambers 10 mL vials with	 CO₂ CH₄ N₂O 	 Samples taken T₀, T₁₅, T₃₀, T₄₅ Weekly to bi-weekly July to October 2014 Humidity minimized: short deployment time Temperature disturbance: reflective mylar Pressure disturbance: chamber vent tube Sample time 10 am or 3 pm to minimize temporal disturbances





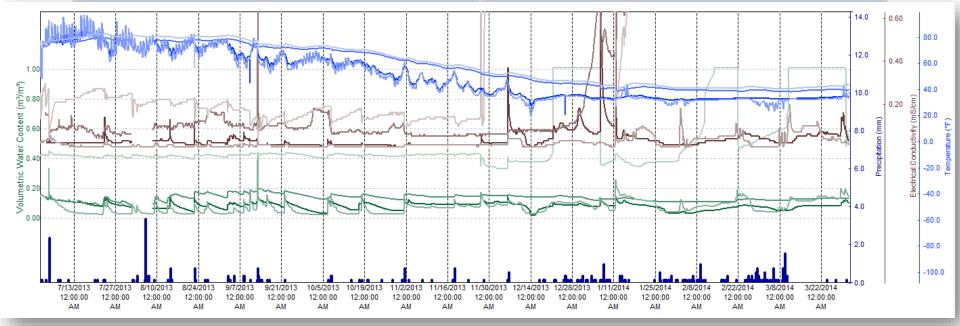




Parkin and Venterea (2010)

Methods: Soil Conditions

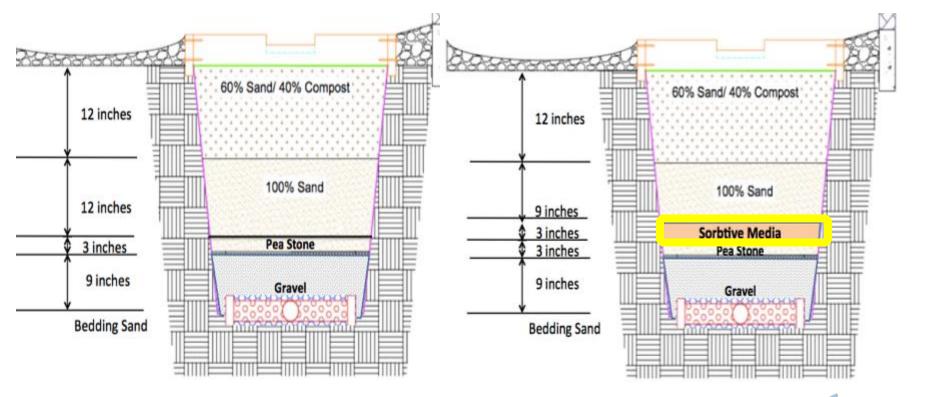
Equipment	Parameter
Decagon Probes (depths of 2" and 2') High Resolution Rain Gauge	Soil temperature Moisture Conductivity Rainfall



Comparing Soil Media Treatments

Conventional Media (CM)

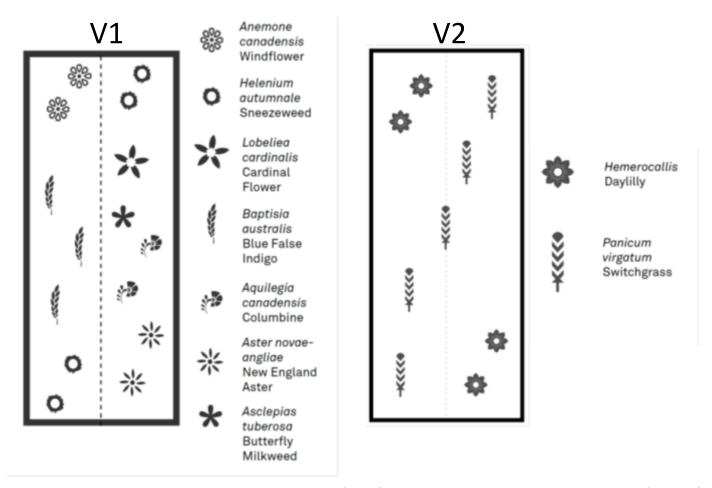
Sorbtive Media [™] (SM)







Comparing Vegetation Treatments



Planting Configuration: Vegetation Palette 1 (left) and Vegetation Palette 2 (right) (Diagram created by S. Hurley and A. Zeitz, unpublished).

Plant Pallet 1: High Species Diversity (7)

Latin Name	Common Name	
Aesclepius incarnata	Butterflyweed, Milkweed 'Tuberosa'	
Anemone canadensis	Windflower	
Aquilegia canadensis	Columbine	
Aster novae-angliae	New England Aster 'Purple Dome'	
Baptisia australis	Blue False Indigo 'Caspian' and 'Midnight Prairiebliss'	
Helenium autumnale	Sneezeweed 'Red + Gold'	
Lobeliea cardinalis	Cardinal Flower	

Plant Pallet 2: Low Species Diversity (2)

Hemerocallis spp.	Daylilies 'Stella d'Oro'
Panicum virgatum	Switch Grass 'Shenandoah'



Vegetation Planted: May 2013



Low Diversity (2 species) vs. High Diversity (7 species)

Established Vegetation: August 2013



Low Diversity (2 species) vs. High Diversity (7 species)

Vegetation 1 (V1)



Vegetation 2 (V2)



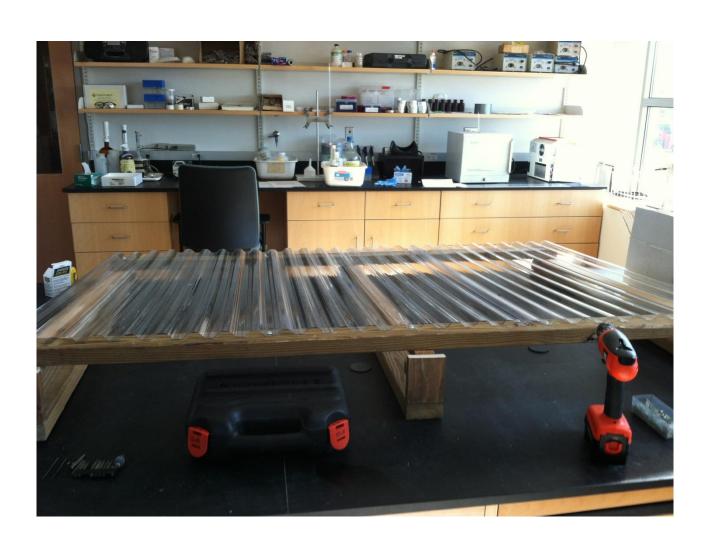
Precipitation Treatments, CM20 and SM60





- 1. Precipitation was added with a simulation device = rain pan
- 2. Runoff was added by increasing the size of the drainage area
- CM20 received 20% more precip via rain pan + drainage area 20% larger than CM
- SM60 received 60% more precip via rain pan + drainage area 60% larger than SM

Simulating Precipitation



Simulating Precipitation





Methods: Greenhouse Gas Emissions

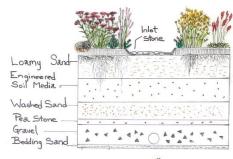
- 1. Measured bi-weekly May-October
- 2. CO_{2} , CH_{4} , $N_{2}O$ three locations per plot, (T_{0}, T_{15}, T_{30})
- 3. Inorganic soil N, moisture, temperature, and bulk density, as covariates for N₂O fluxes

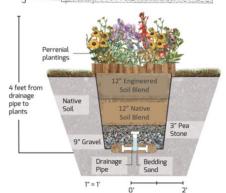


Conclusions

- 1) Bioretention consistently reduce peak stormwater flow rates and volumes.
- 2) Non-labile nutrient and sediment removal is considerable as a result of physical filtration.
- 3) Deep rooted, fine textured roots (*Panicum Virgatum*) provided greater soil stability and access to soil nutrients throughout the profile.
- 4) Organic amendments (compost) added labile nutrient mass loads which far exceed loads from stormwater from a medium traffic paved road surface, and need to be limited.
- 5) Sorbtive Media™ was extremely effective at removing SRP, and may have influenced nitrate removal, although mechanisms are not fully understood.
- 6) Nitrate reduction through extended retention time in an anaerobic zone can provide significant denitrification but optimal conditions necessary are yet to be determined.
- 7) Increased precipitation and runoff may have been linked to increased transport of fine sediment, and partial clogging of the underdrain, but may be site specific.
- 8) Bioretention cells were a small source of N₂0 but not likely significant in the global context.
- 9) Bioretention could be a small sink for CH₄ if the media above the saturated zone is aerobic, but warrants further research given different depths and saturation durations.

Future Research Needs



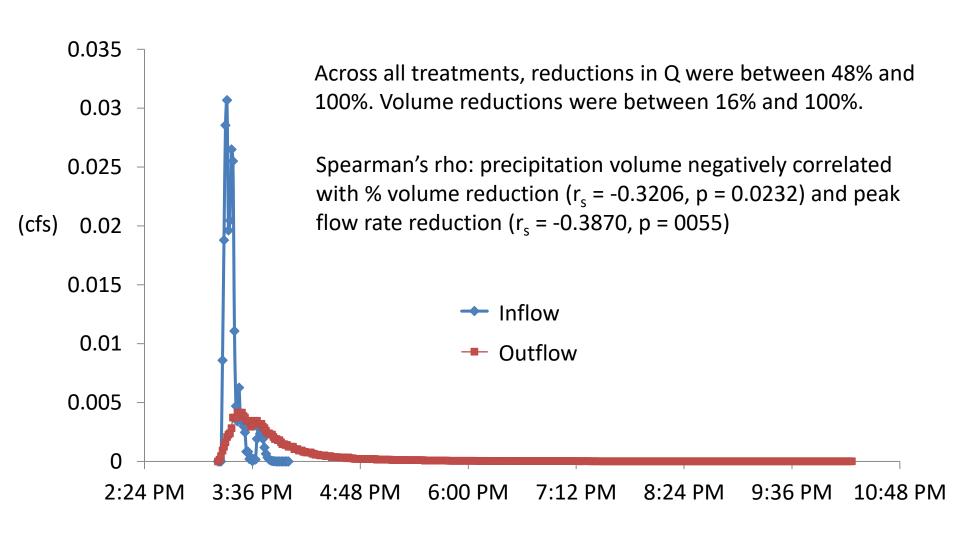




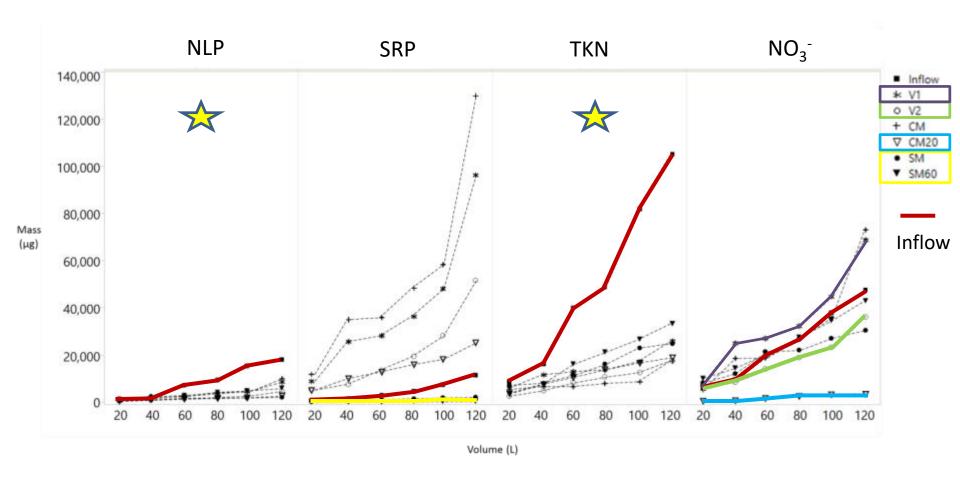
- 1. Chemical characteristics of soil media to minimize soluble N and P contributions (compost, mulch, soil), but achieve target infiltration rate?
- 2. Retention time, carbon requirements for thorough denitrification in different medias?
- 3. Planting options to achieve maximium soil stability and pollutant uptake, given soil conditions (#1) above?

Images: Drawing: A. Cording (2016) Unpublished, (Middle) Cording, A., Hurley, S., Adair, E., Ross, D. (In Preparation). *Evaluating critical bioretention designs features in the context of climate change*. (Bottom) 2012 Nature Education, Conservation Research Institute, Heidi Natura.

Results: Flow Rate Reduction Performance

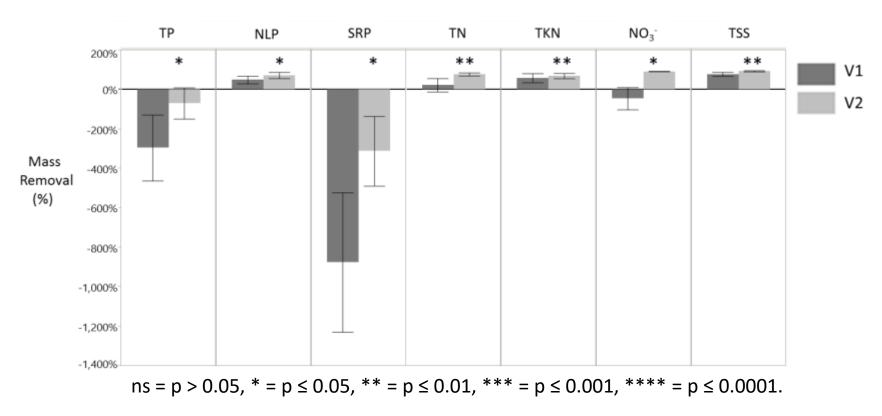


Results: Mass Removal within Each Treatment



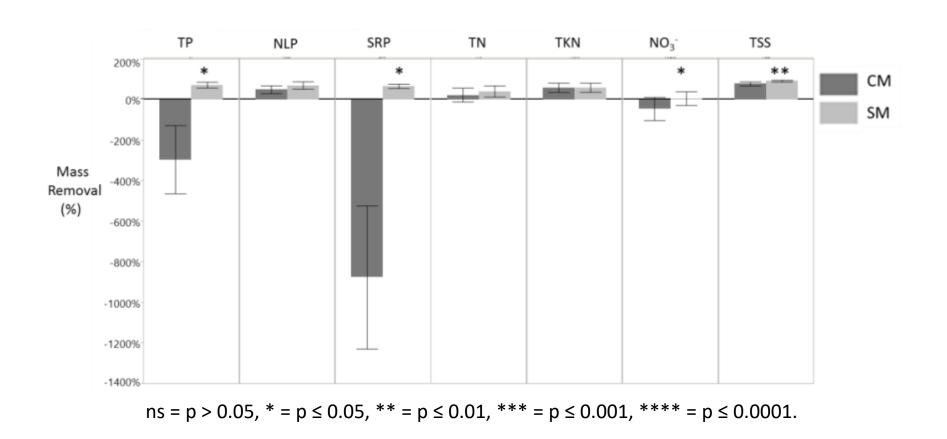
The SRP mass load was significantly increased from inflow to outflow in all treatments, except those containing Sorbtive Media (i.e., SM and SM60).

Results: Outflow Mass between Vegetation Treatments



Paired t-test (n = 6) results indicate that outflow mass from V2 was significantly lower than V1 for all constituents

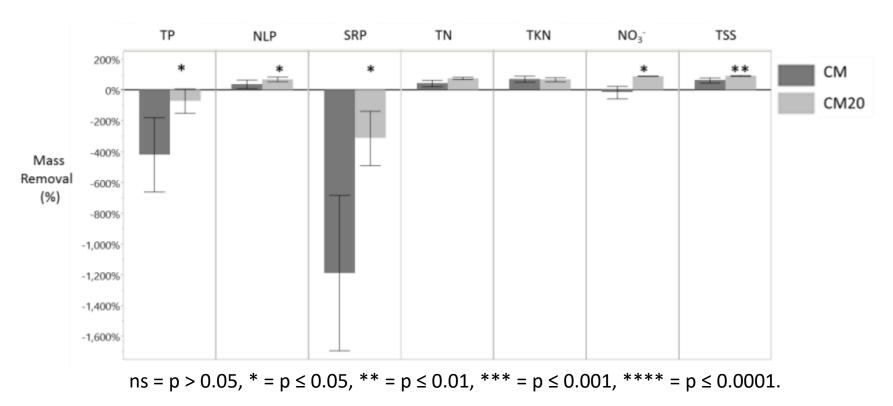
Results: Outflow Mass Between Soil Media Treatments



Outflow mass from SM was lower than the CM for all constituents except NLP and TKN, which were equal between treatments

Results:

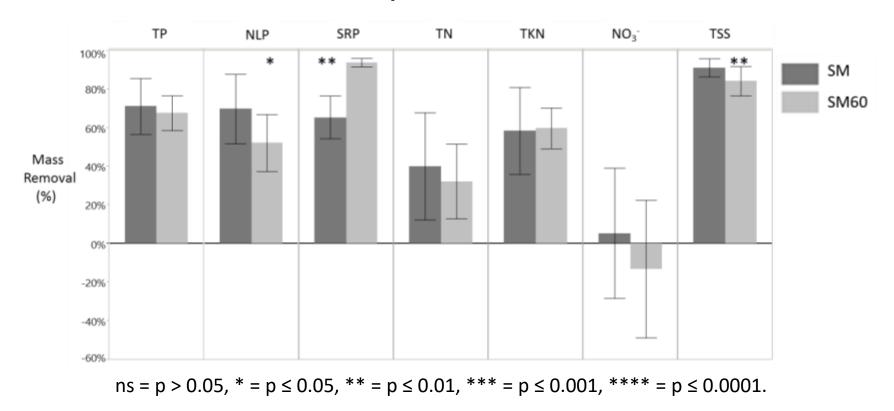
Outflow Mass Between Climate Change Treatments: 20% Increase in Precipitation to Conventional Media



Outflow mass from CM20 was lower than CM for all constituents except TKN, which was found to be equal between treatments

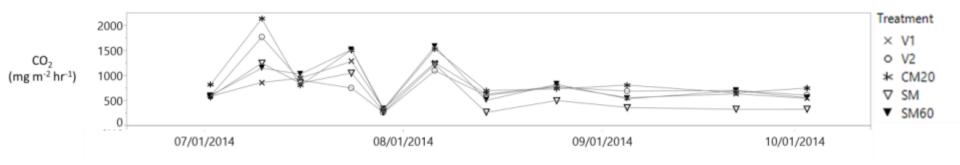
Results:

Outflow Mass Between Climate Change Treatments: 60% Increase in Precipitation to Sorbtive Media™



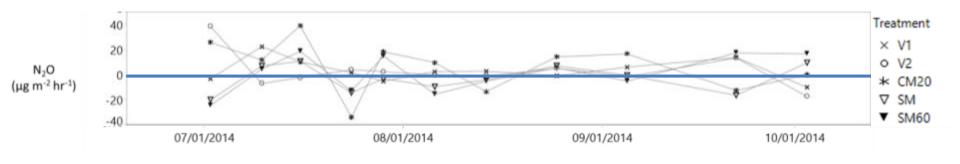
Outflow SRP mass from the SM60 was lower than the SM NLP and TSS mass from SM60 was higher than from SM TKN or NO₃⁻ mass equal between treatments

Results (CO₂): GHG Emissions by Treatment



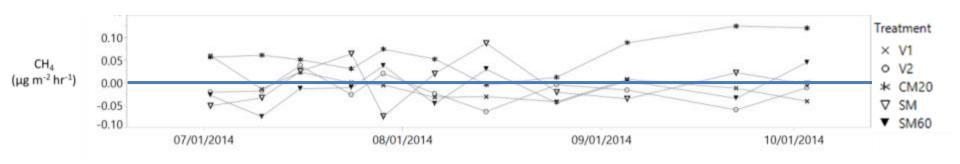
- CO_2 emissions (n = 77), minimum 251 mg m⁻² hr⁻¹ and max 2,650 mg m⁻² hr⁻¹
- Adviento-Borbe et al. (2010) CO_2 ranged 13 mg m⁻² hr⁻¹ to 1,015 mg m⁻² hr⁻¹
- CO_2 positively correlated with soil temperature ($r_s = 0.2545$, p = 0.0255)
- CO_2 negatively correlated with antecedent precip ($r_s = -0.5333$, p < 0.0001) and water filled pore space ($r_s = -0.5400$, p = 0.0065).
- CO_2 from SM60 was greater than SM (t (10) = 4.17, p = 0.0019)

Results (N₂O): GHG Emissions by Treatment



- N_2 O emissions ranged (n = 77) from -33.94 µg m⁻² hr⁻¹ to 65.80 µg m⁻² hr⁻¹
- Grover et al. (2013) found N_2O emission 13.8 μ g m⁻² hr⁻¹ to 65.6 μ g m⁻² hr⁻¹
- The SM was a sink for N_2O overall, with an average (n = 11) of -3.06 μ g m⁻² hr⁻¹

Results (CH₄): GHG Emissions by Treatment



- CH₄ emissions ranged (n = 77) from $-0.1014 \,\mu g \, m^{-2} \, hr^{-1}$ to $0.1259 \,\mu g \, m^{-2} \, hr^{-1}$
- All treatments were a small sink for CH₄ on average (n = 11) except CM20
- CM20 emissions (0.0608 μ g m⁻² hr⁻¹) greater than CM (t (10) = 3.64, p = 0.0046)
- Smith et al. (2003) predict that CH_4 emissions less than 1.6 µg m⁻² hr⁻¹ where depth to saturation > 50 cm, due to negative correlation with depth to "groundwater".

Discussion: Vegetation Treatments

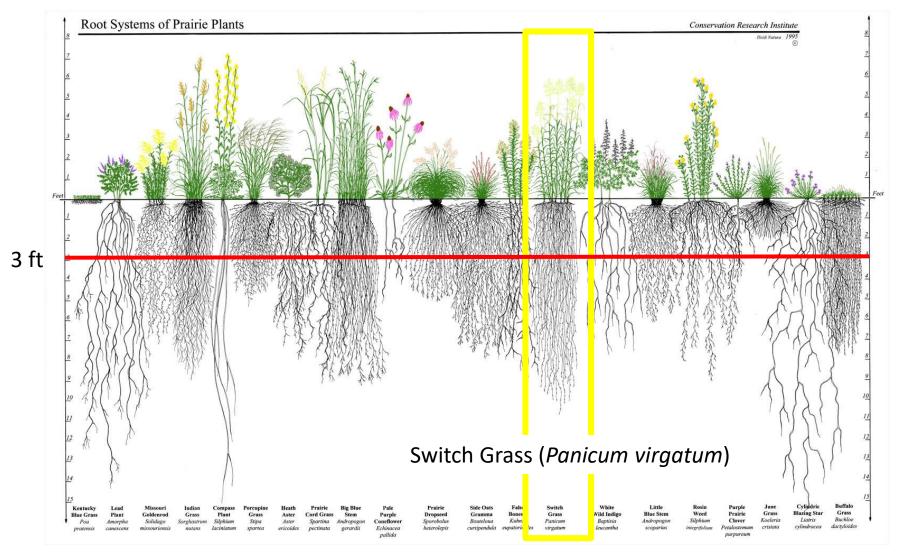
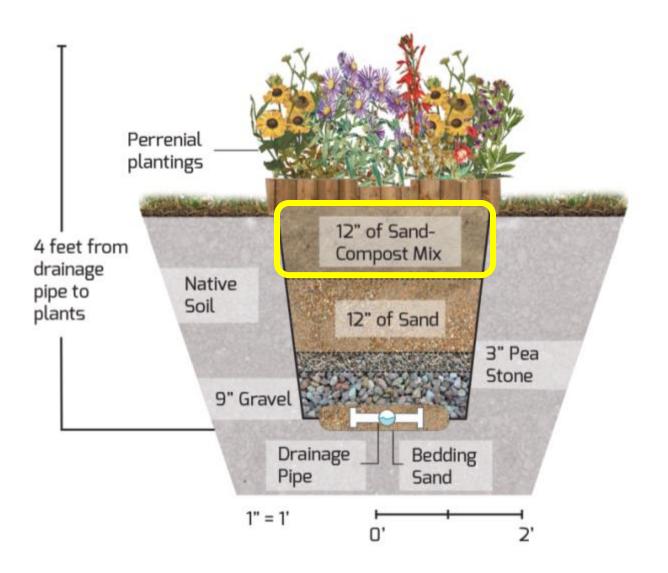


Image Source: Conservation Research Institute; Mann et al. (2013)

Discussion: Conventional Bioretention Design

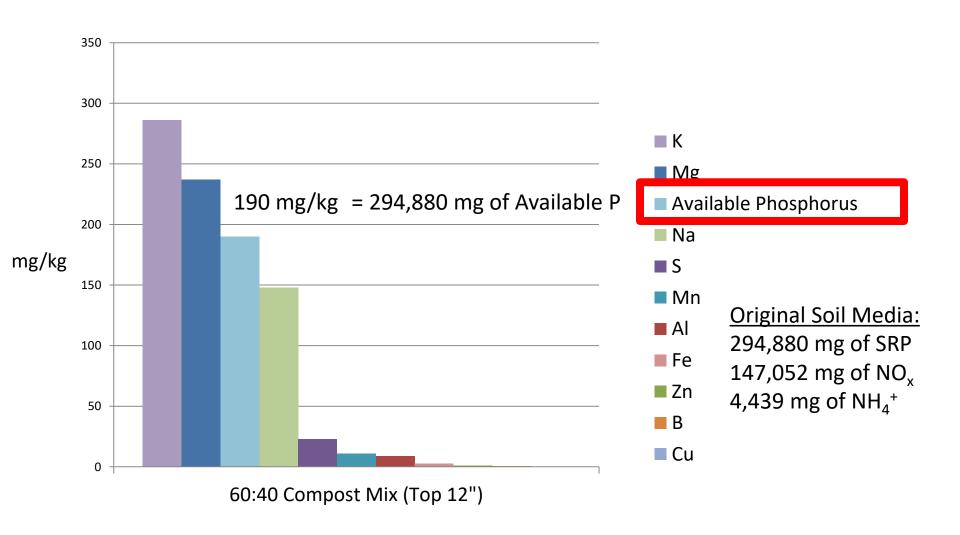


Recommended By:

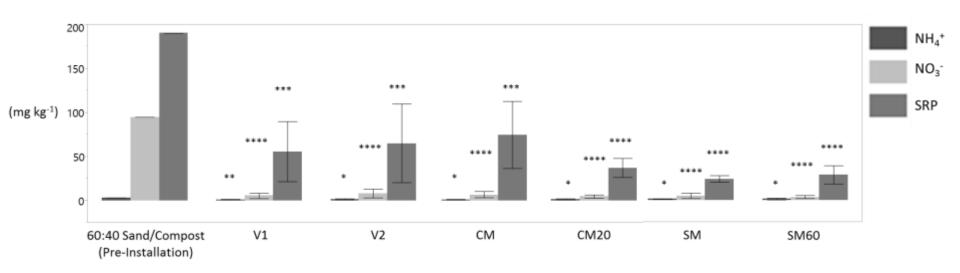
- 1. Vermont Agency of Natural Resources (2002)
 - Washington
 State University
 Pierce County
 Extension (2012)
 - Center for Watershed Protection

Image Credit: Hurley, S., Zeitz, G., (unpublished)

Conventional Bioretention Design: 60:40 Sand Compost Mix

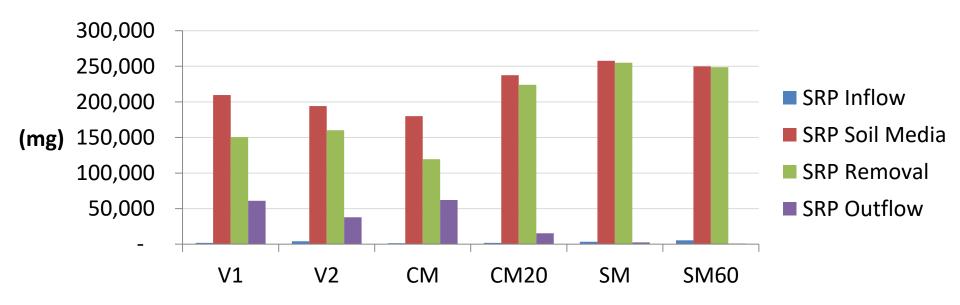


Comparing Nutrient Content in 60:40% Sand and Compost Mixture from Pre-Installation to Average After Two Years



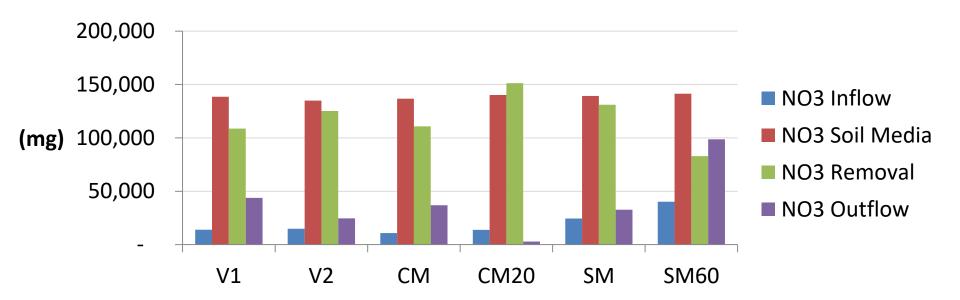
Dunnett's Control: NH_4^+ , NO_3^- , and SRP significantly decreased from the original pre-installation mix after two years, in all treatments (n = 7)

SRP Mass Balance



- In the first two years of installation (n = 7) the SRP content decreased by between 66% (201 g) and 87% (257 g) across all treatments.
- Stormwater runoff contributed between 1% and 2% of the total SRP load to the cells, with the remainder coming from the compost mixture.

NO₃ Mass Balance

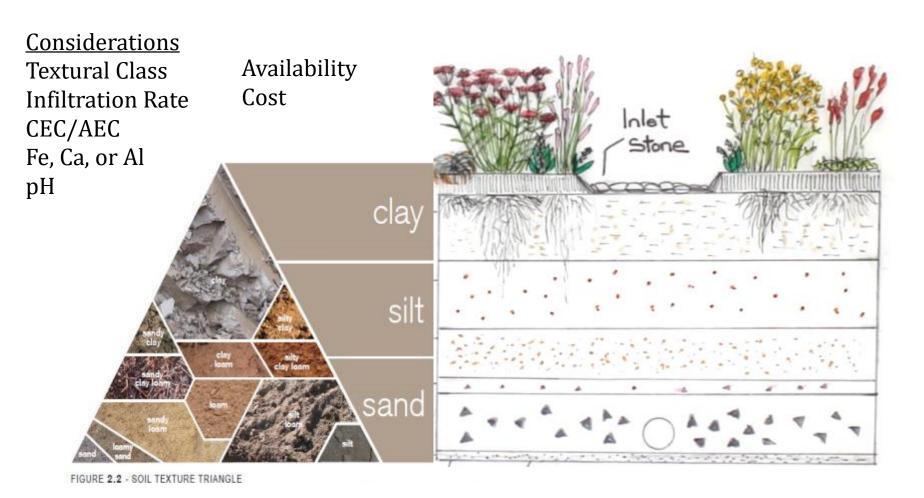


- In the first two years of installation (n = 7) NO_3^- decreased between 92% (135 g) and 96% (141 g).
- NO₃⁻ mass from stormwater contributed between 9% and 22% of the total load.

Mass Balance: SRP and NO₃⁻

- Of the total SRP and NO₃⁻ mass released from the compost and stormwater, approx. 70% was found to be removed by vegetation in V1 and 30% was released to the effluent.
- 80% of the mass load was removed by plant uptake in V2, releasing 20% to the outflow.
- Approximately 1% of the SRP from stormwater + compost mixture was released to the effluent from SM and SM60.

Effective Bioretention Requires the Right Soils



Source: University of Arkansas Community Design Center

Cation Exchange Capacity



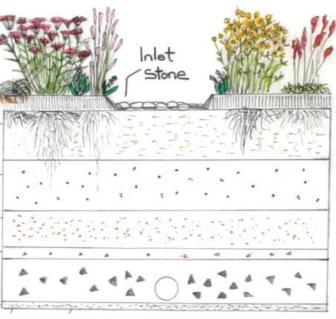
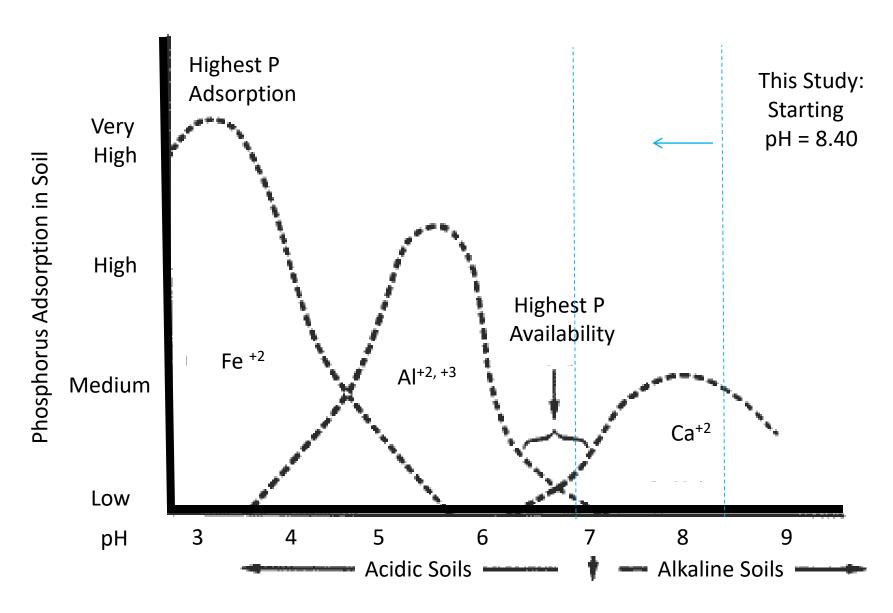
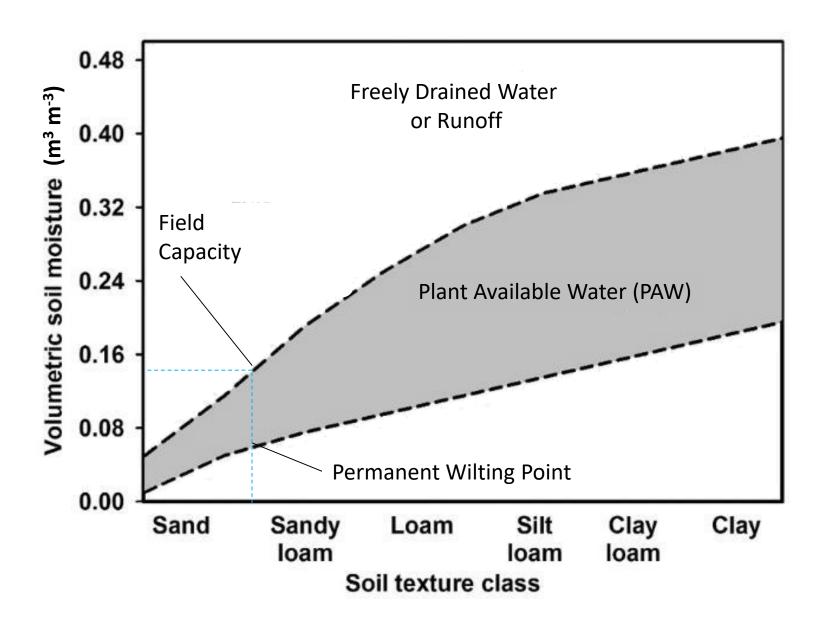


Table 1. Soil textures and CEC (Sonon et al. 2014).

Soil Texture	CEC (meq/100g)
This Study	6.30
Sand	1-5
Fine Sandy Loam	5-10
Loam	5-15
Clay Loam	15-30
Clay	> 30

Phosphorus Sorption and pH





Average Outflow Concentrations Compared to the Literature

Parameter	This Study	Literature		Reference
NLP	18 μg L ⁻¹ (CM20) to 53 μg L ⁻¹ (CM)	40 – 800 μg L ⁻¹	\Rightarrow	Hunt et al. (2006)
SRP	164 μg L ⁻¹ (CM20) to 568 μg L ⁻¹ (CM)	210 – 670 μg L ⁻¹	V	Geosyntec (2008)
	4 μg L ⁻¹ (SM60) to 24 μg L ⁻¹ (SM)	140 μg L ⁻¹	\Rightarrow	Chardon et al. (2005) (Iron Coated Sand)
		< 10 μg L ⁻¹	$\overline{\checkmark}$	O'Neill and Davis (2011) (WW Treat. Residual)
TKN	149 μg L ⁻¹ (CM20) to 376 μg L ⁻¹ (SM)	1,240 – 1,780 μg L ⁻¹	$\stackrel{\wedge}{\Longrightarrow}$	Geosyntec (2008)
NO ₃ -	44 μg L ⁻¹ (CM20) to 464 μg L ⁻¹ (SM60)	300 – 400 μg L ⁻¹	V	Dietz and Clausen (2006)
TSS	3.03 mg L ⁻¹ (CM20) to 10.20 mg L ⁻¹ (CM)	15 – 33 mg L ⁻¹	\Rightarrow	Geosyntec (2008)

Publications:

Cording, A., Hurley, S., Whitney, D. (**Submitted**) Monitoring methods and designs for evaluating bioretention performance. Journal of Environmental Engineering.

Cording, A., Hurley, S., Adair, E., Ross, D. (In Preparation). Evaluating critical bioretention designs features in the context of climate change.

Cording, A. (In Preparation). Investigating pollutant mass mobilization and speciation during the stormwater first flush.















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